

SOLUTIONS TO TUTORIAL EXAMPLES

CHAPTER 19

Note: the standard formulae $M = PL/4$ and $M = wL^2/8$, applied in some of the following examples, are derived and explained in Chapter 16.

Question 1

$$M_{\max} = PL/4 = 5 \times 4/4 = 5 \text{ kNm} = 5.0 \times 10^6 \text{ Nmm.}$$

$$Z = bd^2/6 = 75 \times 300^2/6 = 1\,125\,000 \text{ mm}^3.$$

$$\text{Bending stress } \sigma = M/z = (5.0 \times 10^6 \text{ Nmm}) / (1.125 \times 10^6 \text{ N/mm}^2) = 4.44 \text{ N/mm}^2.$$

Question 2

$$M_{\max} = wL^2/8 = 25 \times 3^2/8 = 28.1 \text{ kNm} = 28.1 \times 10^6 \text{ Nmm.}$$

$$I = \frac{BD^3}{12} - \frac{bd^3}{12}$$

$$I = \frac{100 \times 200^3}{12} - \frac{85 \times 160^3}{12}$$

$$I = (66.67 \times 10^6) - (29.01 \times 10^6) = 37.66 \times 10^6 \text{ mm}^4.$$

Part (a)

Bending stress:

$$\sigma = \frac{My}{I} = \frac{28.1 \times 10^6 \times 100}{37.66 \times 10^6} = 74.6 \text{ N/mm}^2$$

Maximum bending stress in the beam = 74.6 N/mm².

Part (b)

Radius of curvature:

$$R = \frac{EI}{M} = \frac{205 \times 10^3 \times 37.66 \times 10^6}{28.1 \times 10^6}$$

$$R = 274\,743.77 \text{ mm} = 274.7 \text{ metres.}$$

Part (c)

$$\sigma = \frac{My}{I} = \frac{28.1 \times 10^6 \times 80}{37.66 \times 10^6} = 59.7 \text{ N/mm}^2.$$

Note: top of web is 80 mm up from neutral axis position.

Question 3

$$M = 0.50 \text{ kNm} = 0.50 \times 10^6 \text{ Nmm.}$$

$$I = \pi \left(\frac{D^4}{64} - \frac{d^4}{64} \right) = \frac{\pi}{64} (50^4 - 44^4) = 122,812 \text{ mm}^4.$$

$$\sigma = \frac{My}{I} = \frac{0.50 \times 10^6 \times 25}{122,812} = 101.8 \text{ N/mm}^2.$$

Question 4

$$M_{\max} = wL^2/8 = 15 \times 5^2/8 = 46.9 \text{ kNm} = 46.9 \times 10^6 \text{ Nmm}$$

$$I = \frac{BD^3}{12} - \frac{bd^3}{12}$$

$$I = \frac{150 \times 350^3}{12} - \frac{90 \times 290^3}{12}$$

$$I = (0.536 \times 10^9) - (0.183 \times 10^9) = 353 \times 10^6 \text{ mm}^4.$$

Part (a)

$$\sigma = \frac{My}{I} = \frac{46.9 \times 10^6 \times 175}{353 \times 10^6} = 23.3 \text{ N/mm}^2.$$

Part (b)

$$R = \frac{EI}{M} = \frac{20 \times 10^3 \times 353 \times 10^6}{46.9 \times 10^6} = 150,533 \text{ mm} = 150.5 \text{ metres.}$$

Question 5

$$M_{\max} = wL^2/8 = 4 \times 4^2/8 = 8 \text{ kNm} = 8 \times 10^6 \text{ Nmm.}$$

Part (a)

$$I = \frac{BD^3}{12} - \frac{bd^3}{12}$$

$$I = \frac{100 \times 250^3}{12} - \frac{90 \times 230^3}{12}$$

$$I = 10^6(130.2 - 91.3) = 38.9 \times 10^6 \text{ mm}^4.$$

Part (b)

Maximum bending moment = 8 kNm (see above).

Part (c)

$$\sigma = \frac{My}{I} = \frac{8 \times 10^6 \times 125}{38.9 \times 10^6} = 25.7 \text{ N/mm}^2.$$

Part (d)

$$\text{Strain } \varepsilon = \sigma/E = (25.7 \text{ N/mm}^2) / (205 \times 10^3 \text{ N/mm}^2) = 1.25 \times 10^{-4} = 0.000125.$$

Question 6

Zone	b	d	A	y	Ay	h	Ah ² (×10 ⁶)	I = bd ³ /12 (×10 ⁶)
1	200	20	4000	10	40 000	87.5	30.63	0.13
2	20	280	5600	160	896 000	62.5	21.88	36.59
Sum			9600		936 000		52.51	36.72

Part (a)

$$\bar{y} = \frac{\sum(Ay)}{\sum A} = \frac{936,000}{9,600} = 97.5 \text{ mm.}$$

So, using the top edge of the section as datum:

$$y_1 = 10 \text{ mm}$$

$$y_2 = 160 \text{ mm}$$

$$h_1 = (97.5 - 10) = 87.5 \text{ mm}$$

$$h_2 = (160 - 97.5) = 62.5 \text{ mm}$$

Part (b)

$$I = \sum(Ah^2) + \sum\left(\frac{bd^3}{12}\right)$$

$$I = (52.51 + 36.72) \times 10^6 = 89.2 \times 10^6 \text{ mm}^4.$$

Part (c)

$$y_{\text{bottom}} = (300 - 97.5) = 202.5 \text{ mm.}$$

$$\sigma = \frac{My}{I}$$

$$\sigma_{\text{TOP}} = \frac{75 \times 10^6 \times 97.5}{89.2 \times 10^6} = 82.0 \text{ N/mm}^2.$$

$$\sigma_{\text{BTM}} = \frac{75 \times 10^6 \times 202.5}{89.2 \times 10^6} = 170.3 \text{ N/mm}^2.$$

The maximum bending stress is the greater of the above two figures: 170.3 N/mm².

Question 7

Note that the two flanges are different.

Part (a)

Zone	b	d	A	y	Ay	h	Ah ² (×10 ⁶)	I = bd ³ /12 (×10 ⁶)
1	100	10	1000	5	5000	80	6.40	0.008
2	10	160	1600	90	144 000	5	0.04	3.41
3	80	10	800	175	140 000	90	6.48	0.007
Sum			3400		289 000		12.92	3.425

$$\bar{y} = \frac{\sum(Ay)}{\sum A} = \frac{289,000}{3,400} = 85 \text{ mm}$$

So, using the top edge of the section as datum:

$$\begin{aligned} y_1 &= 5 \text{ mm} \\ y_2 &= 90 \text{ mm} \\ y_3 &= 175 \text{ mm} \end{aligned}$$

$$\begin{aligned} h_1 &= (85 - 5) = 80 \text{ mm} \\ h_2 &= (90 - 85) = 5 \text{ mm} \\ h_3 &= (175 - 85) = 90 \text{ mm}. \end{aligned}$$

Part (b)

$$I = \sum(Ah^2) + \sum\left(\frac{bd^3}{12}\right)$$

$$I = (12.92 + 3.425) \times 10^6 = 16.345 \times 10^6 \text{ mm}^4.$$

Part (c)

$$\sigma = \frac{My}{I}$$

$$y_{\text{TOP}} = 85 \text{ mm}$$

$$y_{\text{BTM}} = (180 - 85) = 95 \text{ mm.}$$

$$\sigma_{\text{TOP}} = \frac{50 \times 10^6 \times 85}{16.345 \times 10^6} = 260 \text{ N/mm}^2$$

$$\sigma_{\text{BTM}} = \frac{50 \times 10^6 \times 95}{16.345 \times 10^6} = 290.6 \text{ N/mm}^2.$$

The maximum bending stress is the greater of the above two figures: 290.6 N/mm².